## Benha University <br> Faculty of Engineering at Shoubra Civil Engineering Department Third Year Civil, Structures

Final $2^{\text {nd }}$ Term Exam
Date: 31 / 5 / 2017 Irrigation \& Drainage Engineering CVS 325
Duration: 3 hours

- Answer all the following questions.
- No. of Questions: 4
- Illustrate your answers with sketches when necessary.
- Total Mark: 100 Marks
- The exam consists of 2 pages.


## Question (1) $\quad(\mathbf{1 5}+\mathbf{1 0}=\mathbf{2 5}$ Marks)

A- State True or False \& Correct the False:

1) In semi-arid regions, the available rainfall is not sufficient for the plants' growth.
2) The total volume of water in the world is varying due to climate changes.
3) Capillary water is useful for the plant.
4) Excess water in the soil is the moisture above W.P.
5) Two-turn irrigation rotation must be used when cotton is cultivated.

B- In the figure:

1) State the type of the ground slope?
2) Fix the required constructions on the figure?
3) What is the minimum value for $Y$ ?
4) State the suitable i for the water line?

5) If W.L $=(9.00)$ at Km 0.0 , what is the required W.L in the branch?

## Question (2) (25 Marks)

A branch canal has a length of 15 km , serves an area of 16,500 Feddan, and feeds 3 distributary canals. The land is cultivated as $40 \%$ rice and $55 \%$ Sharaki. The data are in the following table:

| Distributary | Location (L: Left) |  | Land Levels for Distributary Canals at Km: |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Canal |  |  | 0.0 | 1.0 | 2.0 | 3.0 | 4.0 |
| C 1 | 2.0, L | 5000 | (12.00) | (11.95) | (11.90) | (11.80) | (11.70) |
| C 2 | 6.0, L | 4000 | (11.60) | (11.55) | (11.50)/(10.50) | (10.45) | --- |
| C 3 | 10.0, L | 5000 | (11.40) | (11.20) | (11.00) | (10.80) | --- |

1. For a suitable irrigation rotation, sketch a plan for the branch canal and its distributary canals showing the required constructions?
2. Draw the synoptic diagram ONLY for the distributary canal C 1 for lift irrigation?
3. Calculate the area served for design at different sections of the branch canal, (compensation ratio $=20 \%$ ) ?
4. Determine the discharges at different sections of the branch canal, (F.W.D. $=50$ $\left.\mathrm{m}^{3} / \mathrm{Fed} / \mathrm{day}\right)$ ?
5. What is the discharge at km 9.0 of the branch canal?

Question (3) (9+16 = $\mathbf{2 5}$ Marks)
A- Design the cross section at km 5.0 of a branch drain, (A.S. $=20,000$ Feddan, D.F. $=15$ $\mathrm{m}^{3} / \mathrm{Fed} /$ day, $\left.\mathrm{i}=12 \mathrm{~cm} / \mathrm{km}, \mathrm{Z}=1.5 \& \mathrm{~b}=1.5 \mathrm{y}\right)$ ?

B- The figure shows the cross section at km 3.0 of a branch canal that has a discharge of $13 \mathrm{~m}^{3} / \mathrm{s}$ and $\mathrm{i}=10 \mathrm{~cm} / \mathrm{km}$.

1) At km 9.0 of the branch canal, find the bank level so that cut = fill?
2) Draw a typical cross section of the branch canal at km 9.0?
3) Find the velocity at km 9.0 of the branch canal?
4) Discuss this value of the velocity?


Question (4) (25 Marks)

## A Model Answer

$\underline{\text { Question (1) }} \underline{(15+10=25 \text { Marks) }}$
A-

| No | The Statement | T/F | Correction |
| :---: | :--- | :--- | :--- |
| 1 | In semi-arid regions, the available rainfall is not sufficient for the <br> plants' growth. | T | --- |
| 2 | The total volume of water in the world is varying due to <br> llimate changes. | F | constant |
| 3 | Capillary water is useful for the plant. | T | --- |
| 4 | Excess water in the soil is the moisture above $\underline{\text { W.P. }}$ | F | F.C. |
| 5 | Two-turn irrigation rotation must be used when cotton is <br> cultivated. | F | rice |

B-

1) The ground has steep slope.
2) The required constructions are shown on the figure.
3) The minimum value for $Y$ is 1.25 m
4) The suitable slope, $i=30 \mathrm{~cm} / \mathrm{km}$
5) If W.L $=(9.00)$ at Km 0.0 , the required W.L in the branch is (8.80)


## Question (2) (25 Marks)

1. Two - turn irrigation rotation:


## 2. Synoptic diagram for the distributary canal C1:


3. \& 4. The area served for design \& discharges at different sections of the branch canal:

| Location | AS, Feddan |  | AS \& Compensation,Feddan |  | AS Design, Feddan | $\begin{gathered} \text { Discharge, } \mathrm{m}^{3} / \mathrm{s} \\ \mathrm{Q}=\mathrm{AS}_{\text {Design }} * \frac{(50 * 1.15)}{24 * 60 * 60} \end{gathered}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Km | Turn A | Turn B | A+0.2B | B+0.2A |  |  |
| 2, L | $\underline{9,000}$ | 7,500 | 10,500 | 9,300 | 10,500 | 7.04 |
|  | 4,000 | 7,500 | 5,500 | 8,300 | 8,300 | 5.56 |
| 6, L | 4,000 | 7,500 | 5,500 | 8,300 | 8,300 | 5.56 |
|  | 0 | 7,500 | 1,500 | 7,500 | 7,500 | 5.03 |
| 10, L | $\underline{0}$ | 7,500 | 1,500 | 7,500 | 7,500 | 5.03 |
|  | 0 | 2,500 | 500 | 2,500 | 2,500 | 1.68 |

5. The discharge at km 9.0 of the branch canal: $5.03 \mathrm{~m}^{3} / \mathrm{s}$

Question (3) $\quad(\mathbf{9 + 1 6}=\mathbf{2 5}$ Marks)
A- Design the cross section at km 5.0 of a branch drain:
$\mathrm{Q}=$ A.S. $\times$ D.F. $=\frac{20,000 * 15}{24 * 60 * 60} \quad \therefore \mathrm{Q}=3.47 \mathrm{~m}^{3} / \mathrm{sec}$
Trapezoidal section, $\mathrm{z}=1.5 \quad \therefore \mathrm{z}: 1=3: 2$

$\mathrm{A}=\mathrm{by}+\left[2 * \underset{2}{(1 / 2)} * \mathrm{y}_{2} * 1.5 \mathrm{y}\right]=\mathrm{b} y+1.5 \mathrm{y}^{2}$
$\& P=b+2\left(y^{2}+2.25 y^{2}\right)^{1 / 2}=b+3.61 y$

$$
\mathrm{b}=1.5 \mathrm{y}
$$

$\therefore \mathrm{A}=1.5 \mathrm{y}^{2}+1.5 \mathrm{y}^{2}=3 \mathrm{y}^{2}$
$\& P=1.5 y+3.61 \mathrm{y}=5.11 \mathrm{y}$
$\therefore \mathrm{R}=\frac{\mathrm{A}}{\mathrm{P}}=\frac{3 \mathrm{y}^{2}}{5.11 \mathrm{y}}=0.587 \mathrm{y}$
$\mathrm{Q}=\mathrm{A} * \mathrm{v}=(1 / \mathrm{n}) * \mathrm{R}^{3 / 2} * \mathrm{~S}^{1 / 2} * \mathrm{~A}$

$$
\mathrm{S}=\mathrm{i}=12 / 10^{-5} \quad \& \quad 1 / \mathrm{n}=33
$$

$3.47=33 *(0.587)^{2 / 3} * y^{2 / 3} *\left(12^{*} 10^{-5}\right)^{1 / 2} * 3 y^{2}$
$\therefore \mathrm{y}^{8 / 3}=4.57 \quad \therefore \mathrm{y}=1.77 \mathrm{~m} \quad \therefore \mathrm{~b}=2.66 \mathrm{~m}$
Take $\mathrm{b}_{\mathrm{m}}=2.5 \mathrm{~m}$

$$
\mathrm{A}_{\text {calculated }}=\mathrm{A}_{\mathrm{m}}
$$

$\therefore \mathrm{by}+1.5 \mathrm{y}^{2}=\mathrm{b}_{\mathrm{m}} \mathrm{ym}_{\mathrm{m}}+1.5 \mathrm{y}_{\mathrm{m}}{ }^{2}$
$(2.66 * 1.77)+1.5^{*}(1.77)^{2}=2.5 y_{m}+1.5 y_{m}{ }^{2}$
$1.5 \mathrm{ym}^{2}+2.5 \mathrm{y}_{\mathrm{m}}-9.41=0$
$\mathrm{ym}^{2}+1.67 \mathrm{y}_{2}-6.27=0$
$\mathrm{y}=\frac{-\mathrm{b} \pm\left[(\mathrm{b})^{2}-\left(4 * \mathrm{a}^{*} \mathrm{c}\right)\right]^{1 / 2}}{2 * \mathrm{a}}$
$\therefore y_{\mathrm{m}}=-1.67 \pm\left[(1.67)^{2}-\left(4^{*} 1^{*}-6.27\right)\right]^{1 / 2} \quad \therefore \mathrm{y}_{\mathrm{m}}=1.8 \mathrm{~m}$

## B-

1) At km 9.0 of the branch canal,

The levels are as shown in figure ( $\mathrm{i}=10 \mathrm{~cm} / \mathrm{km}$ ).
Bank level = Berm level +y
For simplicity, take $1 / 2$ section as shown in figure.
$\mathrm{A}_{\mathrm{Cut}}=(2.8 * 2)+(2 * 1 / 2 * 3)=8.6 \mathrm{~m}^{2}$
$A_{\text {Fill }}=(6 * y)+(1 / 2 * 2 y * y)=6 y+y 2 m^{2}$

$y^{2}+6 y=8.6$
$y^{2}+6 y-8.6=0$
$y=\frac{-b \pm\left[(b)^{2}-(4 * a * c)\right]^{1 / 2}}{2 * a}$
$\therefore y=\frac{-6 \pm\left[(6)^{2}-\left(4^{*} 1^{*}-8.6\right)\right]^{1 / 2}}{2 \times 1}$
$\therefore \mathrm{y}=1.2 \mathrm{~m}$
Bank level $=(9.40)+1.2=(10.60)$
2) The typical cross section of the branch canal at km 9.0 :

3) $\mathrm{A}=(5.6 * 1.5)+(2 * 1 / 2 * 2.25 * 1.5)=11.78 \mathrm{~m}^{2}$
$\therefore \mathrm{v}=\mathrm{Q} / \mathrm{A}=13 / 11.78=1.1 \mathrm{~m} / \mathrm{s}$
4) $\mathrm{v}>0.9 \mathrm{~m} / \mathrm{s}$, So, it will cause scour.

For non-silting non-scouring conditions, $0.3<\mathrm{v}<0.9$
We have to reduce the velocity by increasing the water area.

